

# ELEVATING SLEEVE PILE REPAIR SYSTEM

## Calculations and Drawings

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***Disclaimer:***

***The following calculations and drawings  
(Pages 1 thru 12) have been stamped based  
on the design parameters included and for  
the jurisdiction as noted upon the seal.***



# NUCOR®

## SKYLINE

### Pipe



# PIPE

Diameter in mm	Wall in mm	Weight lb/ft kg/m	Inside Diameter in mm	Cross Sectional Area in <sup>2</sup> cm <sup>2</sup>	Total Area of Pile in <sup>2</sup> cm <sup>2</sup>	Internal Area in <sup>2</sup> cm <sup>2</sup>	Internal Volume ft <sup>3</sup> /ft m <sup>3</sup> /m	External Coating Area ft <sup>2</sup> /ft m <sup>2</sup> /m	Moment of Inertia in <sup>4</sup> cm <sup>4</sup>	Section Modulus in <sup>3</sup> cm <sup>3</sup>	Radius of Gyration in cm	Maximum Available Yield Strength of A252		
												ERW	SW	R&W
												ksi (MPa)		
12 ¾ 324	0.250 6.350	33.41 49.71	12.25 311.2	9.82 63.34	1277 823.7	1179 760.4	0.819 0.076	3.34 1.02	191.8 7984	30.09 493.1	4.42 11.23	70 483	Not Available	Not Available
	0.312 7.925	41.48 61.73	12.13 308.0	12.19 78.65	1277 823.7	115.5 745.1	0.802 0.075	3.34 1.02	235.9 9,819	37.00 606.4	4.40 11.17	70 483		
	0.375 9.525	49.61 73.82	12.00 304.8	14.58 94.06	1277 823.7	113.1 729.7	0.785 0.073	3.34 1.02	279.3 11,630	43.82 718.0	4.38 11.12	70 483		
	0.394 10.008	52.04 77.44	11.96 303.8	15.29 98.67	1277 823.7	112.4 725.0	0.780 0.073	3.34 1.02	292.2 12,161	45.83 751.0	4.37 11.10	70 483		
	0.406 10.312	53.58 79.73	11.94 303.2	15.74 101.6	1277 823.7	111.9 722.1	0.777 0.072	3.34 1.02	300.2 12,500	47.09 771.7	4.37 11.09	70 483		
	0.500 12.700	65.48 97.44	11.75 298.5	19.24 124.1	1277 823.7	108.4 699.6	0.753 0.070	3.34 1.02	361.5 15,050	56.7 929.4	4.34 11.01	70 483		
	0.525 13.335	68.61 102.1	11.70 297.2	20.16 130.1	1277 823.7	107.5 693.6	0.747 0.069	3.34 1.02	377.4 15,710	59.20 970.0	4.33 10.99	70 483		
	0.687 17.450	88.59 131.8	11.38 289.0	26.04 168.0	1277 823.7	101.6 655.7	0.706 0.066	3.34 1.02	475.1 19,780	74.53 1,221	4.27 10.85	70 483		
14 356	0.219 5.563	32.26 48.01	13.56 344.5	9.48 61.17	153.9 993.1	144.5 932.0	1.003 0.093	3.67 1.12	225.1 9,371	32.16 527.1	4.87 12.38	70 483	Not Available	Not Available
	0.250 6.350	36.75 54.68	13.50 342.9	10.80 69.67	153.9 993.1	143.1 923.5	0.994 0.092	3.67 1.12	255.3 10,630	36.47 597.7	4.86 12.35	70 483		
	0.312 7.925	45.65 67.94	13.38 339.8	13.42 86.56	153.9 993.1	140.5 906.6	0.976 0.091	3.67 1.12	314.4 13,090	44.91 736.0	4.84 12.30	70 483		
	0.375 9.525	54.62 81.28	13.25 336.6	16.05 103.6	153.9 993.1	137.9 889.6	0.958 0.089	3.67 1.12	372.8 15,520	53.25 872.6	4.82 12.24	70 483		
	0.438 11.125	63.50 94.50	13.12 333.3	18.66 120.4	153.9 993.1	135.3 872.7	0.939 0.087	3.67 1.12	429.5 17,880	61.36 1,005	4.80 12.19	70 483		
	0.500 12.700	72.16 107.4	13.00 330.2	21.21 136.8	153.9 993.1	132.7 856.3	0.922 0.086	3.67 1.12	483.8 20,140	69.11 1,132	4.78 12.13	70 483		
	0.625 15.875	89.36 133.0	12.75 323.9	26.26 169.4	153.9 993.1	127.7 823.7	0.887 0.082	3.67 1.12	588.5 24,500	84.08 1,377	4.73 12.02	70 483		
16 406	0.250 6.350	42.09 62.64	15.50 393.7	12.37 79.81	201.1 1,297	188.7 1,217	1.310 0.122	4.19 1.28	383.7 15,970	47.96 785.9	5.57 14.15	70 483	60 414	Not Available
	0.312 7.925	52.32 77.86	15.38 390.6	15.38 99.21	201.1 1,297	185.7 1,198	1.289 0.120	4.19 1.28	473.2 19,700	59.16 969.4	5.55 14.09	70 483	60 414	
	0.375 9.525	62.64 93.21	15.25 387.4	18.41 118.8	201.1 1,297	182.7 1,178	1.268 0.118	4.19 1.28	562.1 23,400	70.26 1,151	5.53 14.04	70 483	60 414	
	0.406 10.312	67.68 100.7	15.19 385.8	19.89 128.3	201.1 1,297	181.2 1,169	1.258 0.117	4.19 1.28	605.0 25,180	75.62 1,239	5.52 14.01	70 483	60 414	
	0.500 12.700	82.85 123.3	15.00 381.0	24.35 157.1	201.1 1,297	176.7 1,140	1.227 0.114	4.19 1.28	731.9 30,470	91.49 1,499	5.48 13.93	70 483	60 414	
	0.625 15.875	102.7 152.9	14.75 374.7	30.19 194.8	201.1 1,297	170.9 1,102	1.187 0.110	4.19 1.28	893.5 37,190	111.7 1,830	5.44 13.82	70 483	60 414	
18 457	0.250 6.350	47.44 70.59	17.50 444.5	13.94 89.94	254.5 1,642	240.5 1,552	1.670 0.155	4.71 1.44	549.1 22,860	61.02 999.9	6.28 15.94	60 414	60 414	Not Available
	0.312 7.925	59.00 87.79	17.38 441.4	17.34 111.9	254.5 1,642	237.1 1,530	1.647 0.153	4.71 1.44	678.2 28,230	75.36 1,235	6.25 15.89	60 414	60 414	
	0.375 9.525	70.66 105.1	17.25 438.2	20.76 134.0	254.5 1,642	233.7 1,508	1.623 0.151	4.71 1.44	806.6 33,570	89.63 1,469	6.23 15.83	60 414	60 414	
	0.500 12.700	93.54 139.2	17.00 431.8	27.49 177.3	254.5 1,642	227.0 1,464	1.576 0.146	4.71 1.44	1,053 43,840	117.0 1,918	6.19 15.72	60 414	60 414	
	0.625 15.875	116.1 172.8	16.75 425.5	34.12 220.1	254.5 1,642	220.4 1,422	1.530 0.142	4.71 1.44	1,289 53,660	143.2 2,347	6.15 15.61	Not Available	60 414	

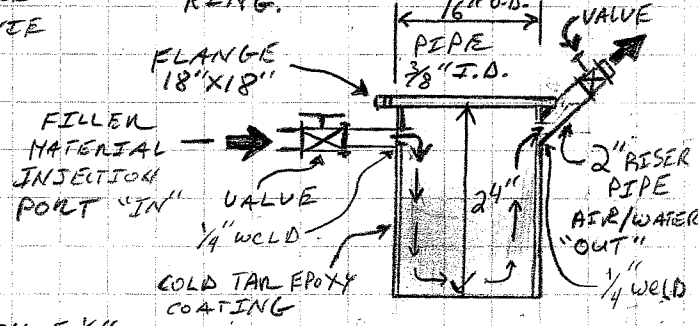
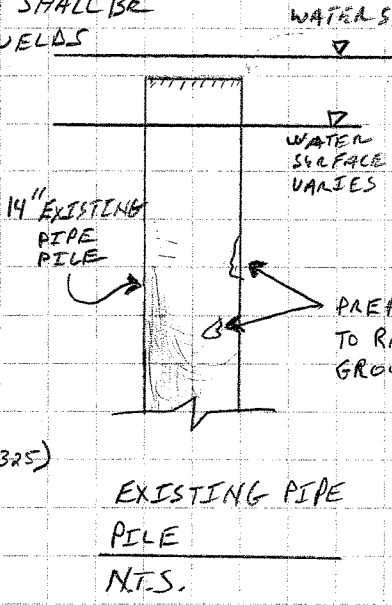
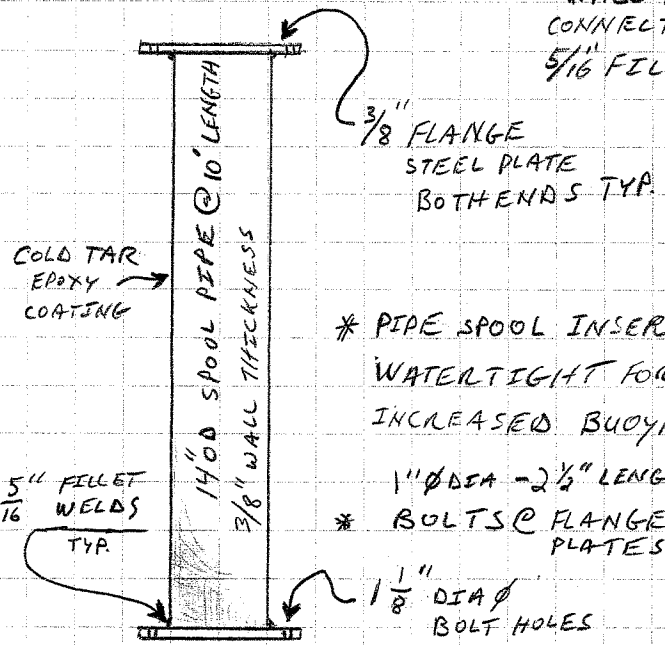
# ELEVATING SLEEVE PILE REPAIR SYSTEM

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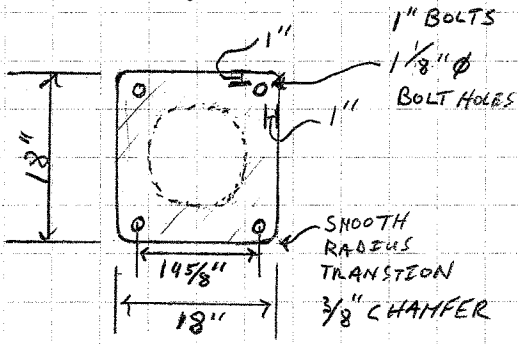
\* ALL FLANGE TO PIPE WELD CONNECTIONS SHALL BE 5/16" FILLET WELDS

\* ALL 1" BOLTS SHALL USE LOCK WASHERS AND SHALL BE HOT DIPPED GALVANIZED TYPICAL.

\* EXISTING PIPE PILE TO RECEIVE PILE CAP RING PRIOR TO GROUT FILL TO ENSURE ALL THREAD ROD EMBEDMENT 6" MIN TOP AND BOTTOM OF PILE CAP RING.

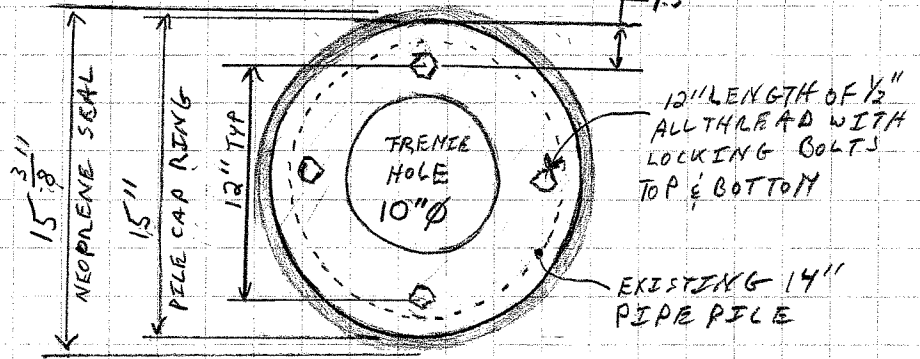
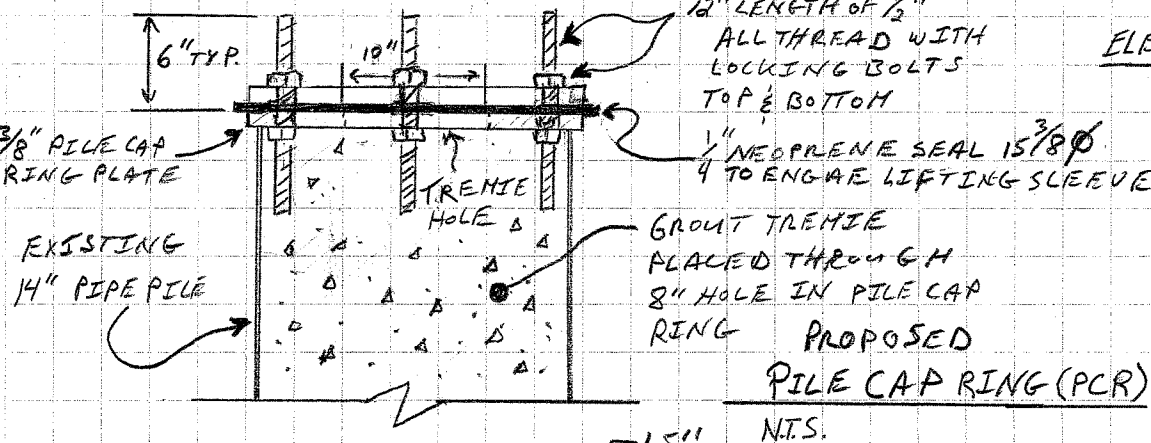


PIPE SPOOL INSERT N.T.S.

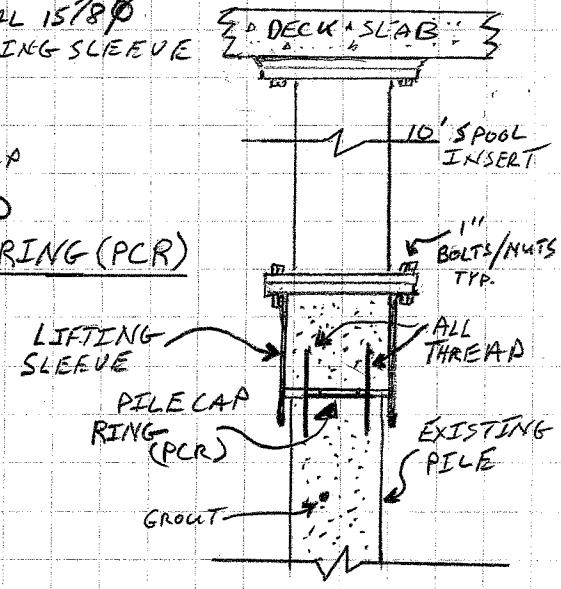


PIPE SPOOL INSERT FLANGE N.T.S.

DATE: 3/16/2023  
DRAWN BY: SET

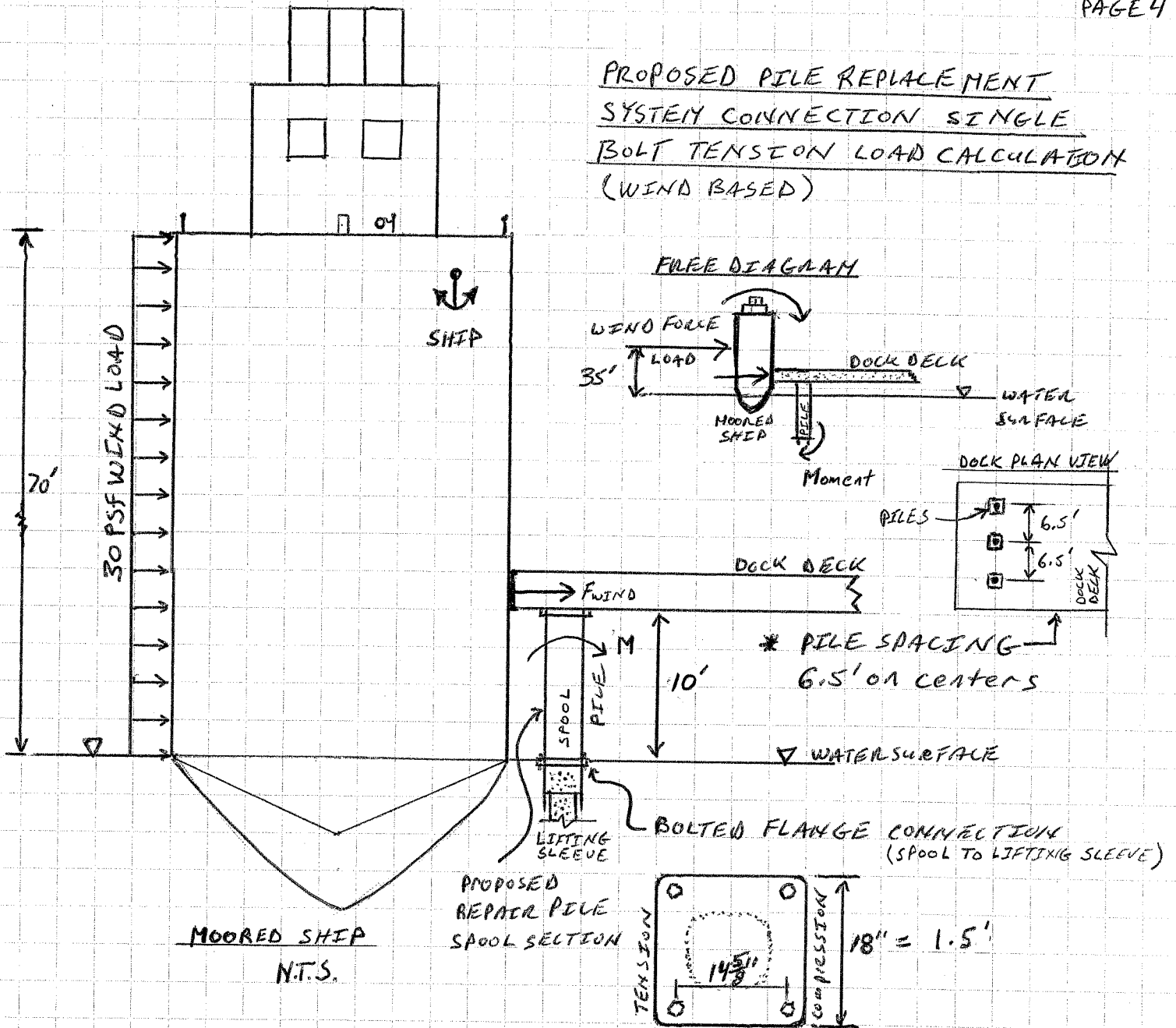


ELEVATING SLEEVE N.T.S.



PROPOSED PILE REPLACEMENT SYSTEM N.T.S.

PROPOSED PILE REPLACEMENT SYSTEM CONNECTION SINGLE BOLT TENSION LOAD CALCULATION (WIND BASED)



CALCULATE WORST CASE SCENARIO ON ONE BOLT LOADING FOR THE BOLTED CONNECTION

DISTRIBUTED LOAD AREA  
 $A = (70') (6.5') = 455 \text{ ft}^2$

CALCULATE WIND FORCE  
 $F_{\text{WIND}} = P(A)$   
 $F_{\text{WIND}} = (30 \text{ lb/ft}^2) (455 \text{ ft}^2)$   
 $F_{\text{WIND}} = 13,650 \text{ lbs}$

WIND LOAD MOMENT  $\downarrow +$   
 $M_{\downarrow+} = 0 = F_{\text{WIND}} (\text{DIST TO CENTER})$   
 $M_{\downarrow+} = (13,650 \text{ lbs}) (10 \text{ ft})$   
 $M_{\downarrow+} = 136,500 \text{ lb-ft}$

SINGLE BOLT TENSION LOAD

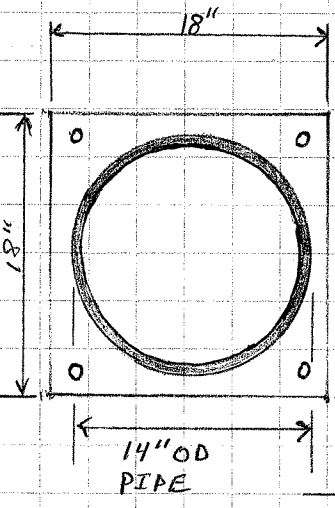
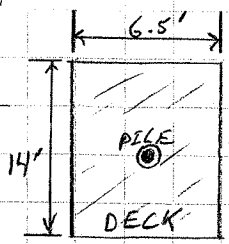
$T = \frac{136,500 \text{ lb-ft}}{1.5'} = 91,000 \text{ lb} \times F.O.S 2.5$   
 $= 227.5 \text{ kips}$

$T_{\text{ONE BOLT}} = 227.5 \div 2 = 114 \text{ kips}$

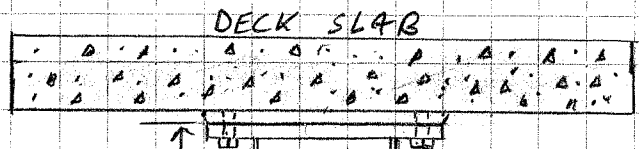
LIVE DECK LOAD  $850 \text{ lb/ft}^2$

WEIGHT LIVE LOAD  
 $W_L = (850 \text{ lb/ft}^2)(9 \text{ ft}^2)$   
 $W_L = 77,350 \text{ lbs}$

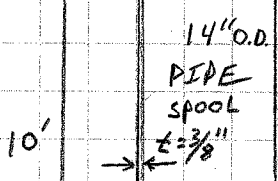
TRIBUTARY LOAD AREA  
 $A_D = 14' \times 6.5'$   
 $A_D = 91 \text{ ft}^2$



PIPE WALL THICKNESS  
 $t = 0.375$



WEIGHT CONCRETE DECK (DEAD LOAD)  
 $W_C = (150 \text{ lb/ft}^3)(14 \text{ ft})(91 \text{ ft}^2)$   
 $W_C = 13,650 \text{ lbs}$



WEIGHT SPOOL (DEAD LOAD)  
 $W_S = 600 \text{ lbs}$

PARAMETERS

- GROSS AREA ( $A_G$ ) =  $16.05 \text{ in}^2$
- MOMENT OF INERTIA ( $I$ ) =  $372.8 \text{ in}^4$
- SECTIONAL MODULUS ( $S$ ) =  $53.25 \text{ in}^3$
- RADIUS OF GYRATION ( $r$ ) =  $4.82 \text{ in}$
- MODULUS OF ELASTICITY ( $E$ )  
 $E = 2.9 \times 10^7 \text{ PSI}$
- $F_y = 42,000 \text{ PSI}$

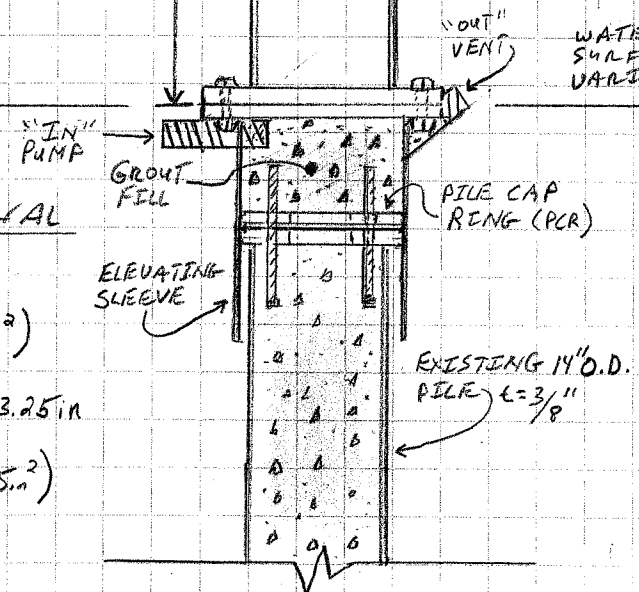
14" PIPE CROSS SECTIONAL AREA

$$A_{14} = \left(\frac{\pi}{4}\right) * (D_o^2 - D_i^2)$$

$$D_o = 14 \text{ in} \quad D_i = 13.25 \text{ in}$$

$$A_{14} = \left(\frac{\pi}{4}\right) * (14 \text{ in}^2 - 13.25 \text{ in}^2)$$

$$A_{14} = 16.052 \text{ in}^2$$



USE AISC CHAPTER E "DESIGN OF MEMBERS FOR COMPRESSION"

\* FIND DESIGN COMPRESSIVE STRENGTH  $\phi_c P_n$  (LRFD)  
 $\phi_c = 0.9$

\* USE TABLE USER NOTE E1.1 - SELECTION TABLE FOR THE APPLICATION OF CHAPTER E SECTIONS. DETERMINE LIMITING STATES FOR AXIALLY LOADED PIPE.

\* FOR PIPE SECTION, THE LIMITING STATE IS NOTED AS FLEXURAL BUCKLING (FB) FOR MEMBERS "WITHOUT" SLENDER ELEMENTS & FLEXURAL BUCKLING (FB) OR LOCAL BUCKLING (LB) FOR A MEMBER "WITH" SLENDER ELEMENTS

\* DETERMINE IF THE MEMBER HAS SLENDER ELEMENTS.

REFER TO AISC TABLE B4.1a - "WIDTH TO THICKNESS RATIOS: COMPRESSION ELEMENTS, MEMBERS SUBJECT TO AXIAL COMPRESSION."

\* THE WIDTH TO THICKNESS RATIO =  $\frac{d}{t}$ , UTILIZING SUB-PARAGRAPH 2 "DESIGN WALL THICKNESS FOR HOLLOW STRUCTURAL SECTIONS (HSS) CONSTRUCTED AS ELECTRIC-RESISTANCE-WELDED (ERW) PIPE. THE DESIGN WALL THICKNESS "t" SHALL BE TAKEN EQUAL TO 0.93 TIMES THE NOMINAL WALL THICKNESS.

$$\text{DIAMETER OF PIPE} = 14''$$

$$\text{WALL THICKNESS PIPE} = 0.375''$$

$$\text{WIDTH TO RATIO THICKNESS} = \frac{d}{0.93(t)} = \frac{14''}{(0.93)(0.375'')} = 40.14$$

\* THE LIMITING WIDTH TO THICKNESS RATIO IS GIVEN BY ROW 9 OF THE TABLE AS:

$$(B4.1a) \quad \lambda_r = 0.11 \frac{E}{F_y} = (0.11) \left( \frac{2.9 \times 10^7 \text{ PSI}}{42,000 \text{ PSI}} \right) = 75.95$$

\* COMPARING  $\frac{d}{t} = 40.14 < \lambda_r = 75.95$  THE SPOOL MEMBER IS "NON-SLENDER"

\* THE MEMBER LIMITING STATE IS FLEXURAL BUCKLING (FB) THEREFORE CHAPTER E.3 APPLIES.

\* CHAPTER E.3 "FLEXURAL BUCKLING OF MEMBERS WITHOUT SLENDER ELEMENTS"

NOMINAL COMPRESSIVE STRENGTH ( $P_n$ ) IS DETERMINED

$$\text{BY } P_n = F_c A_g$$

\* FIND THE CRITICAL STRESS  $F_c$

E.3- DETERMINE IF THE EFFECTIVE LENGTH IS LESS THAN OR EQUAL TO:

$$4.71 \sqrt{\frac{E}{F_y}}$$

$$\text{EFFECTIVE LENGTH} = \frac{KL}{r}$$

L = UNBRACED LENGTH  
K = EFFECTIVE LENGTH FACTOR

r = RADIUS OF GYRATION

K - COMES FROM APPEN 7 FOR MEMBER END  
CONDITIONS. MEMBER FIXED AT BOTH ENDS.  
 $K = 0.65$

$$\text{EFFECTIVE LENGTH} = \frac{(0.65)(10\text{ft})(12 \frac{\text{in}}{\text{ft}})}{4.82\text{in}} = 16.18$$

NOW COMPARE:

$$4.71 \sqrt{\frac{2.9 \times 10^5 \text{ PSI}}{42,000 \text{ PSI}}} = 123.76$$

$16.18 \leq 123.76$  CRITICAL STRESS IS AS  
FOLLOWS:

$$F_{cr} = \left(0.685 \frac{F_y}{F_e}\right) F_y$$

$F_e$  = ELASTIC BUCKLING STRESS GIVEN BY:

$$F_e = \frac{\pi^2 E}{\left(\frac{KL}{r}\right)^2} = \frac{\pi^2 (2.9 \times 10^5 \text{ PSI})}{(16.18)^2 \left(\frac{1000\text{lb}}{1\text{KEF}}\right)} = 1093 \text{ KSI}$$

$$F_{cr} = \left(0.685 \frac{42 \text{ KSI}}{1093 \text{ KSI}}\right) 42 \text{ KSI} = 41.39 \text{ KSI}$$

\* NOMINAL COMPRESSIVE STRENGTH  $P_n$

$$P_n = F_{cr} A_g = (41.39 \text{ KSI})(16.05 \text{ in}^2)$$

$$P_n = 664 \text{ KEPS UNFACTORED}$$

FACTORED USING LRFD  $\phi_c = 0.9$

$$\text{FACTORED } P_n = (0.9)(664 \text{ KEPS})$$

$$\underline{\phi_c P_n = 598 \text{ KEPS}}$$



\* DETERMINE ACTUAL LOAD  $P_A$

TRIBUTARY AREA = 91 S.F.

\* SEE PAGE 1  
FIGURE DETAIL

DEAD LOAD

WEIGHT SPOOL PIPE = 600 lbs

WEIGHT CONCRETE = 13,650 lbs

LIVE LOAD

WEIGHT LIVE LOAD = 77,350 lbs

$$P_A = 1.2 D_L + 1.6 L_L = 1.2 (600 \text{ lbs} + 13,650 \text{ lbs}) + 1.6 (77,350 \text{ lbs})$$

$$P_A = 140,860 \text{ lbs} = 140.9 \text{ KIPS}$$

\* DETERMINE FACTOR OF SAFETY F.O.S.

$$F.O.S. = \frac{\phi P_n}{P_A} = \frac{598 \text{ KIPS}}{140.9 \text{ KIPS}} = 4.24$$

$$F.O.S. = 4.24$$

∴ IN CONCLUSION - A 14" DIAMETER X 10 FT LONG,  $\frac{3}{8}$ " WALL PIPE IS SUFFICIENT FOR THE SPOOL PIECE APPLICATION